



## **TECHNICAL NOTE**

Date: 22 September 2021

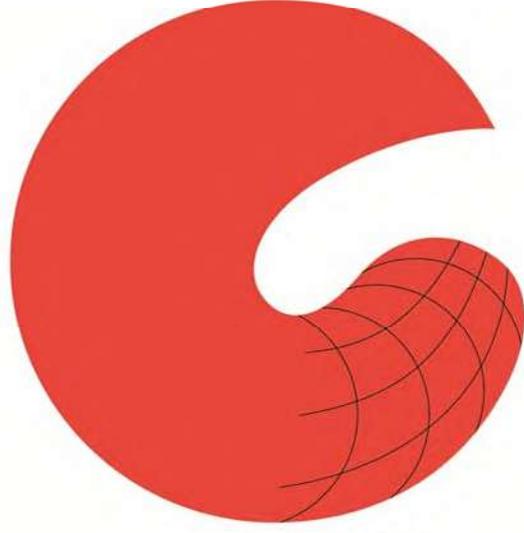
File Ref: SG/VL/P21-2315/01TN Vol. 2 of 3

Project: Land at Impington, Histon, Cambridgeshire

Subject: Summary of Flood Risk and Drainage Matters

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## **APPENDIX D**



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**NIAB, PARK FARM, CAMBRIDGE  
Watercourse Hydraulic Modelling Report**

# NIAB, PARK FARM, CAMBRIDGE

## Watercourse Hydraulic Modelling Report

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**Reference:** JJ/CS/P17-1210/01

**Date:** March 2017

**NIAB, PARK FARM, CAMBRIDGE**  
**Watercourse Hydraulic Modelling Report**

# NIAB, PARK FARM, CAMBRIDGE

## Watercourse Hydraulic Modelling Report

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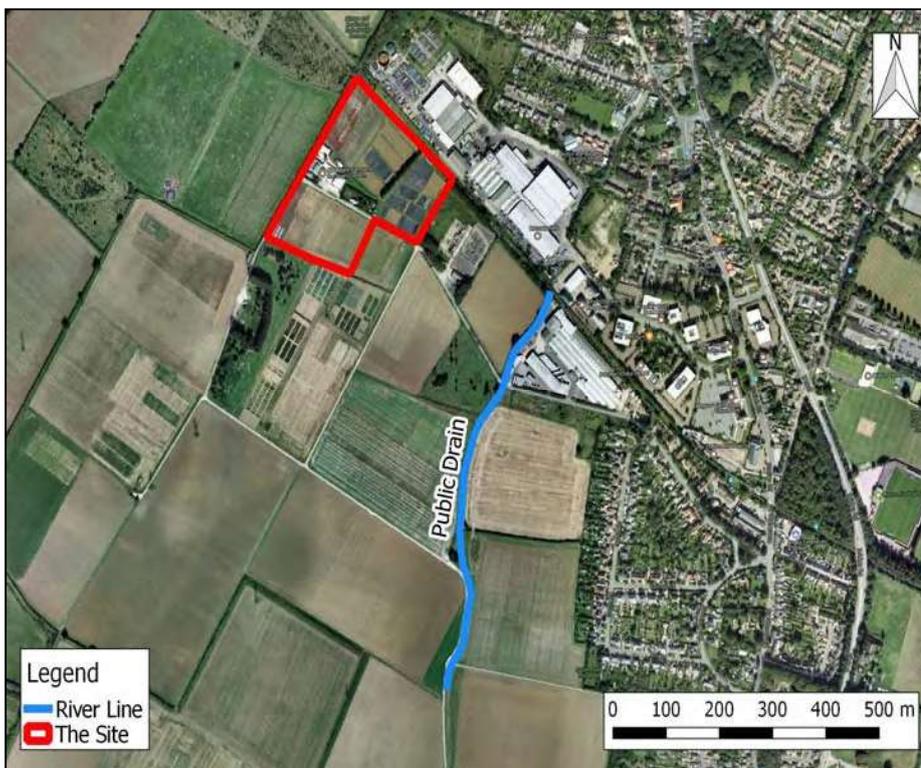
### Registration of Amendments

Revision and Date	Amendment Details	Revision Prepared By	Revision Approved By

## 1.0 INTRODUCTION

### Brief

- 1.1 This report provides a summary of the flood modelling assessment that was carried out by Create Consulting Engineers Ltd on behalf of the National Institute of Agricultural Botany (NIAB) Trust for the proposed development at Park Farm, 1 Villa Rd, Histon, Cambridge, CB24 9NZ.
- 1.2 The unnamed watercourse labelled on OS plans as ‘Public Drain’, located approximately 260 m to the south east of the site, was modelled using HEC-RAS software. The Public Drain and the site location is shown on Figure 1.1.



**Figure 1.1: Location of the Site and Public Drain**

### Project Context

- 1.3 The purpose of this report is to respond to the Environment Agency’s (EA) request for a site specific hydraulic analysis of the watercourse (Public Drain) adjacent to Park Farm. This is to determine the flood risk to the proposed development with an allowance for climate change.
- 1.4 We understand that this hydraulic assessment will be submitted as part of a site specific Flood Risk Assessment for a planning application for development of a new glasshouse, two agricultural style buildings, laboratories, office space, and an additional carpark at Park Farm.

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- 1.5 Architect's Layouts are included at the end of this report.
- 1.6 The Topographic Survey, is included within this report on Drawings ALS75618/OVERVIEW and ALS75618/500/01 to ALS75618/500/04. Relative to ordnance datum, ground levels at the site fall from 14.94 mAOD in the western extent of the site to 11.83 mAOD in the eastern extent of the site. It is to be noted that over the area that is proposed to be developed the ground levels range from 13.22 mAOD to 14.26 mAOD.

### **Constraints and Limitations**

- 1.7 The copyright of this report is vested in Create Consulting Engineers Ltd and the Client, NIAB Estate. The Client, or his appointed representatives, may copy the report for purposes in connection with the development described herein. It shall not be copied by any other party or used for any other purposes without the written consent of Create Consulting Engineers Ltd or the Client.
- 1.8 Create Consulting Engineers Ltd accepts no responsibility whatsoever to other parties to whom this report, or any part thereof, is made known. Any such other parties rely upon the report at their own risk.
- 1.9 The Watercourse Hydraulic Modelling Report addresses the flood risk posed to the proposed development, the extent of which is shown by the site boundary, as indicated by the location plan attached with this report.
- 1.10 This report has been undertaken with the assumption that the site will be developed in accordance with the above proposals without significant change. The conclusions resulting from this study are not necessarily indicative of future conditions or operating practices at or adjacent to the site.
- 1.11 Create Consulting Engineers Ltd has endeavored to assess all information provided to them during this appraisal. The report summarises information from a number of external sources and cannot offer any guarantees or warranties for the completeness or accuracy or information relied upon. Information from third parties has not been verified by Create Consulting Engineers Ltd unless otherwise stated in this report.

## 2.0 HYDROLOGICAL ASSESSMENT

### Watercourse and Catchment Area

- 2.1 The Public Drain drains towards the north combining with various other drains before reaching its confluence with the Beck Brook approximately 2 km to the north west of the site.
- 2.2 The area surrounding the drain is relatively flat and primarily comprises agricultural fields. The watercourse is understood to be fed by a catchment 3.27 km<sup>2</sup> in size to the south of the site. The catchment has been inferred from the FEH Web Mapping Service and verified against a watershed analysis using GRASS software and the Environment Agency LiDAR (Figure 2.1).

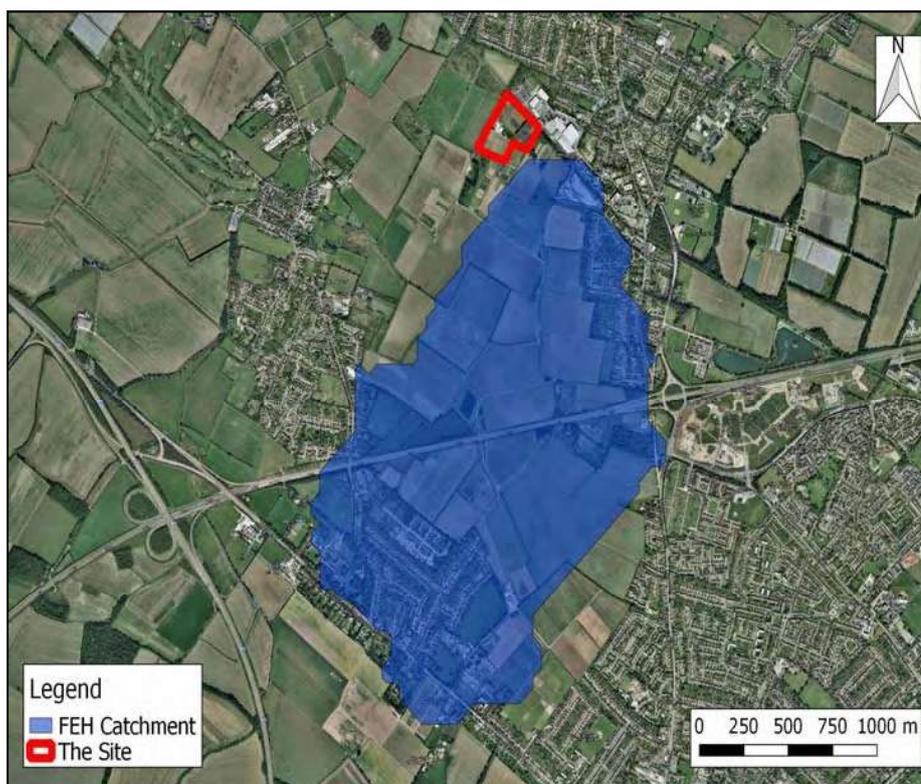


Figure 2.1: Location of the FEH Catchment

### Estimate of Flows in Watercourse

- 2.3 There is no gauged flow data available for the Public Drain and therefore the flow regime has been estimated using both WINFAP 4 and ReFH2 with catchment descriptors extracted from the FEH Web Mapping Service (Appendix A).
- 2.4 The median flow (QMED) for the study catchment (index flood) was calculated as 0.281 m<sup>3</sup>/s from the catchment descriptors using WINFAP 4.
- 2.5 Peak flow estimates produced by ReFH2 and WINFAP 4 are outlined below.

ReFH2

- 2.6 Using the ReFH2 program and the inputs from the FEH online web service (Appendix A), estimates were obtained for the site for various return periods.
- 2.7 The results are displayed in Table 2.1.

WINFAP 4

- 2.8 WINFAP 4 uses flow records from either a single reliable gauged site in the same catchment or from a number of other gauged sites in hydrologically similar catchments to form a pooling group which is subjected to a statistical analysis. Following this the flood growth curve and flood frequency curve generated by the analysis are used to calculate a range of flood flows at the site in question. This was carried out for the watercourse using the inputs from the FEH online web service (Appendix A). A default urban adjustment factor was applied (Urbext2000). The results are displayed in Table 2.1
- 2.9 A sensitivity analysis of the urban adjustment (URBext2000) factor was undertaken. The percentage of impermeable area was determined using aerial imagery and it was concluded that the percentage of impermeable area was approximately half of the default value presented by Urbext2000. The default Urbext2000 value was utilised for calculating flows as this value was more conservative than the calculated value.

Estimate of Bankfull Flow

- 2.10 A sensitivity analysis was carried out to determine the impact of using a bankfull flow estimate for QMED. WINFAP 4 has the functionality to produce a bankfull estimate for QMED.
- 2.11 The bankfull QMED was calculated using both the catchment descriptors and channel dimensions. The channel was measured in several places and an average Bankfull Channel Width of 5.95 was used. The results are displayed in Table 2.1.

Comparison of Flow Results

- 2.12 The results from each method are shown in Table 2.1.

Return Period (Years)	Flows Calculated using ReFH2 (m <sup>3</sup> /s)	Flows Calculated using WINFAP 4 (m <sup>3</sup> /s)	Flows Calculated from Bankfull Analysis (in WINFAP 4) (m <sup>3</sup> /s)
<b>2</b>	0.49	0.28	0.63
<b>5</b>	Not Modelled	0.39	0.87
<b>10</b>	Not Modelled	0.48	1.06

Return Period (Years)	Flows Calculated using ReFH2 (m <sup>3</sup> /s)	Flows Calculated using WINFAP 4 (m <sup>3</sup> /s)	Flows Calculated from Bankfull Analysis (in WINFAP 4) (m <sup>3</sup> /s)
20	0.89	Not Modelled	Not Modelled
25	Not Modelled	0.60	1.33
50	Not Modelled	0.70	1.57
100	1.34	0.83	1.84
200	Not Modelled	0.97	2.16
500	Not Modelled	1.19	2.65
1000	2.51	1.39	3.10

**Table 2.1: Flows Calculated using 1) ReFH2, 2) WINFAP 4, and 3) Bankfull Analysis (Using WINFAP) for Several Return Periods**

2.13 To provide a conservative estimate of the flows in the Public Drain, peak flows from the bankfull analysis will be utilised in the flood model.

#### Climate Change Allowances

2.14 To incorporate the potential future effects of climate change the bankfull flows have been increased accordingly.

2.15 The development is classified as 'less vulnerable' use according to the NPPF. Based on the EA's<sup>1</sup> guidance the 'central' (25%) and 'higher central' (35%) allowances for the Anglian Basin 2070-2115 will be added on to the flows based on a development design life of 100 years. Correspondence with the Environment Agency (28th February to 13th March 2017) confirmed that these allowances are acceptable. The results are displayed in Table 2.1.

Return Period (Years)	Flows Calculated from Bankfull Analysis (in WINFAP 4)	+25% Climate Change	+35% Climate Change
2	0.63	0.78	0.85
5	0.87	1.09	1.18
10	1.06	1.32	1.43
25	1.33	1.66	1.79
50	1.57	1.96	2.11
100	1.84	2.30	2.48
200	2.16	2.69	2.91
500	2.65	3.31	3.58
1000	3.10	3.87	4.18

**Table 2.2: Flows calculated using bankfull analysis (in WINFAP) to include 25% climate change and 35% climate change allowances**

<sup>1</sup> Environment Agency (2016) *Flood Risk Assessments: Climate Change Allowances*.

### 3.0 METHODOLOGY

- 3.1 The hydraulic model has been developed to estimate the flood extent of the Public Drain for the 1 in 100 year event. As the channel is both small and relatively straight a one dimensional model was chosen to represent the channel which has been constructed using HEC-RAS 5.0. A 700m stretch of the drain (identified in Figure 1.1) ending at the Bypass was chosen to best represent water levels at the site. This reach comprises 4 bridges and one culvert.
- 3.2 The model was run in a steady state scenario by using a constant flow input for the 1 in 100 year event (as the pooling analysis generates a peak flow only).

#### Channel Survey Data

- 3.3 A detailed topographical survey was undertaken on 7 March 2017 (Drawings 2219-545-SU01 - 2219-545-SU16). 42 channel cross sections at 20 m intervals were generated using the topographical survey as well as a 3D surface.

#### Model Schematic

- 3.4 A schematic representation of the model is included in Figure 3.1 at the rear of the report.

#### Surface Roughness

##### Channel and Floodplain

- 3.5 The following surface roughness assumptions have been made for the channel and floodplain based on a combination of site photographs and google earth imagery. The values have been obtained from the HEC-RAS reference manual (US Army Corps Engineers, 2016). The Manning’s ‘n’ values are shown in Table 3.1 below.

Geometry	Manning’s ‘n’	Description
Channel	0.35	Clean, straight, full, no rifts or deep pools but more stones and weeds
Floodplain	0.04	Mature field crops

**Table 3.1: Manning’s ‘n’ values used for the channel and floodplain in HEC-RAS**

##### Structures

- 3.6 The four bridges have been modelled using the dimensions, soffit levels, and invert levels provided within the topographical survey. The bridges are clear span with no piers or edges so it was assumed the surface roughness would remain the same as the channel. The culvert located in the northern part of the model has been modelled as an arched culvert. The

following surface roughness assumptions have been made and the Manning’s ‘n’ values are shown in Table 3.2 below.

Geometry	Manning’s ‘n’	Description
Bridges	0.35	Remained the same as channel
Culvert	0.015	Culvert with some debris

**Table 3.2: Manning’s ‘n’ values used for the channel and floodplain in HEC-RAS**

**Contraction and Expansion**

3.7 Contraction and expansion coefficients for the bridges and culverts were obtained from the HEC-RAS manual as shown in Table 3.3 below.

Geometry	Contraction Coefficient	Expansion Coefficient	Description
Bridges	0.3	0.5	Typical bridge
Culvert	0.6	0.8	Abrupt transition

**Table 3.3: Contraction and expansion values used in HEC-RAS**

**Boundary Conditions**

3.8 A bed slope of 0.0008 was used throughout the length of the model, which was calculated (from the topographic survey) as the average gradient of the channel along the section in question.

## 4.0 RESULTS OF MODELLING

- 4.1 The 1 in 100 year event with an allowance for climate change was modelled and the results are shown in Table 4.1 (to the rear of the report). The 100 year with a 35% allowance for climate change scenario has been selected as the design flood event for the development when assessing the flood risk to the development with additional considerations made to the results of sensitivity scenarios carried out.
- 4.2 For the 100 year plus 35% climate change scenario, levels in the channel ranged from 10.87m AOD to 11.63m AOD. Freeboard along the left bank ranges from 10 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.3 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.4 The results show that flow remains in channel along the modelled reach (as shown in the long profile Figure 4.1). The lowest levels of the site are between 12.09 m AOD and 11.84 m AOD in the eastern corner. It is to be noted that over the area that is proposed to be developed the ground levels range from 13.22 mAOD to 14.26 mAOD.
- 4.5 The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level. It is concluded that the development is at a low risk of flooding from the public drain and has sufficient freeboard above the design flood event (100yr + cc) for the lifetime of the development.

### Sensitivity Testing

- 4.6 As requested by the EA several sensitivity tests were undertaken to test the assumptions made within the model. These included an uplift of the following:
- Coefficients used for Manning's 'n' hydraulic roughness
  - Contraction/expansion losses
  - Peak flows
- 4.7 The results are discussed in the following sections.

### Manning's 'n'

- 4.8 A sensitivity test was undertaken using the modelled design flood event (1 in 100 year flow plus 35% climate change) with a 25% increase in the channel and floodplain Manning's 'n' values. This increased the Mannings 'n' values to 0.044 for the channel and 0.05 for the floodplain. Results are displayed in Table 4.2, to the rear of the report.

- 4.9 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.99m AOD to 11.36m AOD. Freeboard along the left bank ranges from -80 mm (exceeding bank level) to 470 mm and along the right bank it ranges from -10 mm to 420 mm.
- 4.10 The results show that the flows primarily remain in channel, however, it overtops within chainage 177 to 0 in one location along the left bank (depth of 80 mm) and one along the right bank (depth of 10 mm).
- 4.11 The lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site will therefore remain dry in this event. Additionally the lowest part of the site with development proposals is at 13.22 m AOD (Barn 03).
- 4.12 The site levels are well above the design flood event peak level of 11.36 mAOD with the proposed developed area (13.22 mAOD) being 1.86m above this level. It is concluded that with a 25% increase in Manning's n, the development remains at a low risk of flooding for the design flood event.

#### Contraction/Expansion Losses

- 4.13 A sensitivity test was undertaken using the modelled design flood event with a 25% increase in the contraction and expansion losses values for the bridges and culvert. Results are displayed in Table 4.3, to the rear of the report
- 4.14 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 580 mm and along the right bank it ranges from 110 mm to 550 mm.
- 4.15 The results show that the flows remain in channel in all locations within chainage 177-0. Additionally, the lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site therefore remains dry during this event.
- 4.16 The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level. It is concluded that with a 25% increase in contraction and expansion losses, the development remains at a low risk of flooding for the design flood event.

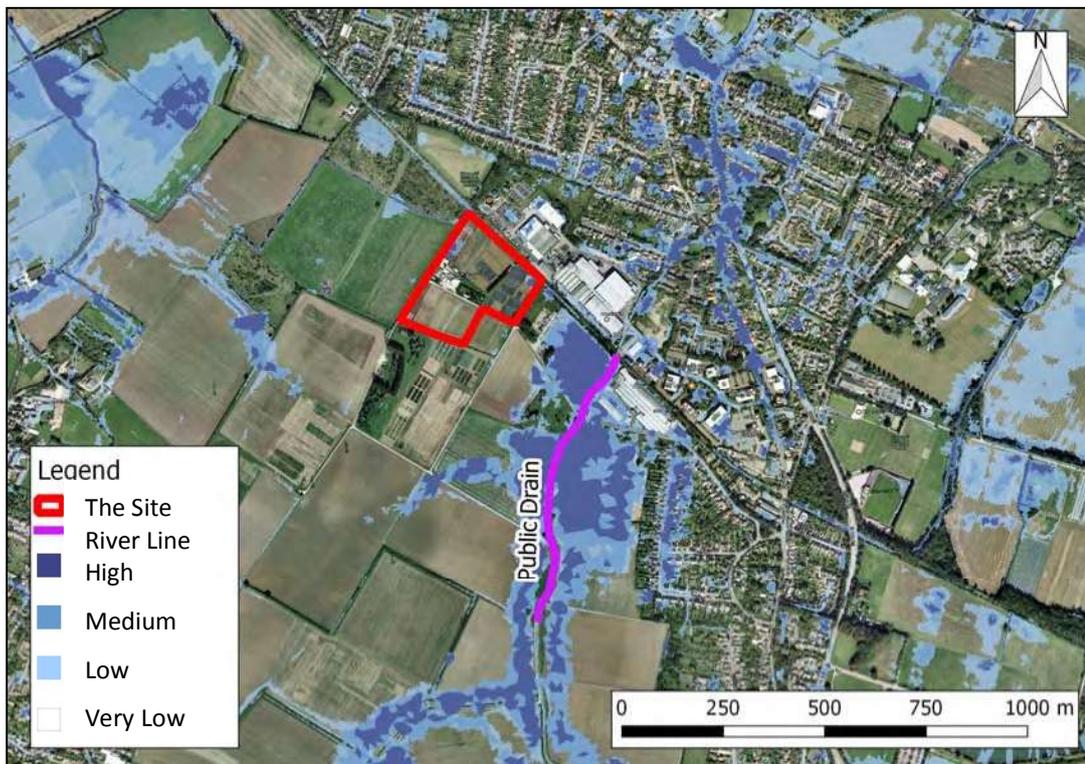
#### Peak Flows

- 4.17 A sensitivity test was undertaken using the design flood event with a 25% increase to the peak flow (roughly equivalent to the 1000yr event). Results are displayed in Table 4.4, to the rear of the report.

- 4.18 In the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.99m AOD to 11.35m AOD. Freeboard along the left bank ranges from -80 mm (exceeding bank level) to 480 mm and along the right bank it ranges from -10 mm to 430 mm.
- 4.19 The results show that the flows remain in channel in all locations within chainage 177-0. Additionally, the lowest levels of the site are between 12.09 m AOD and 11.84 m AOD (in the eastern corner). The site therefore remains dry during this event.
- 4.20 The site levels are well above the design flood event peak level of 11.35 mAOD with the proposed developed area being 1.87m above this level. It is concluded that with a 25% increase in peak flow, the development remains at a low risk of flooding for the design flood event.

**Calibration**

- 4.21 There is no gauged data or data from observed events available for calibration purposes.
- 4.22 It is noted however that the EA Surface Water Flood Maps (Figure 3.1) shows the site to be at ‘very low’ flood risk. This provides a form of validation against the results, showing that the site is not affected.



**Figure 4.2 EA Surface Water Flood Maps**

## 5.0 CONCLUSIONS

- 5.1 This report has provided a summary of the flood modelling assessment that was carried out by Create Consulting Engineers Ltd on behalf of the NIAB Trust for the proposed development at Park Farm, 1 Villa Rd, Histon, Cambridge, CB24 9NZ.
- 5.2 A hydrological assessment was undertaken to calculate the flows for the Public Drain. The bankfull estimate (using WINFAP 4) was used as it was the most conservative estimate, with an addition of 25% and 35% for climate change.
- 5.3 The hydraulic model has been developed to estimate the flood risk to the development from the Public Drain for the 1 in 100 year event with an allowance for climate change. As the channel is both small and relatively straight a one dimensional model was chosen to represent the channel which has been constructed using HEC-RAS 5.0.
- 5.4 The model was based on a detailed topographical survey was undertaken on 7th March 2017 (Drawings 2219-545-SU01 - 2219-545-SU16). 42 channel cross sections at 20 m intervals were generated using the topographical survey as well as a 3D surface.
- 5.5 The results of the model for the 100yr +35% flood event (the chosen design flood event) showed that the cross sections closest to the development (chainage 177 to 0) levels in the channel ranged from 10.87m AOD to 11.23m AOD. Freeboard along the left bank ranges from 40 mm to 590 mm and along the right bank it ranges from 110 mm to 550 mm.
- 5.6 The results showed that flow remains in channel along the modelled reach. The site levels are well above the design flood event peak level of 11.23 mAOD with the proposed developed area (13.22 mAOD) being 1.99m above this level.
- 5.7 As requested by the EA several sensitivity tests were undertaken to test the assumptions made within the model. These included an uplift of the following:
- Coefficients used for Manning's 'n' hydraulic roughness
  - Contraction/expansion losses
  - Peak flows
- 5.8 The results of these sensitivity tests shows the site to not be affected by any of the flood events.
- 5.9 There is no gauged data or data from observed events available for calibration purposes. It is noted, however, that the EA Surface Water Flood Maps shows the site to be at 'very low' flood risk. This provides a form of validation against the results, showing that the site is not affected.

- 5.10 It is concluded that the levels on site are situated sufficiently above flood levels modelled in the public drain. It also concluded that the development will be at a low risk of flooding from the public drain and will have sufficient freeboard above the design flood event (100yr + cc) for the lifetime of the development.

## 6.0 REFERENCES

- i. Environment Agency Surface Water Flood WMS Layers (2014) Available at: <https://data.gov.uk/data/search> (Accessed March 2017).
- ii. US Army Corps Engineers (2016). HEC-RAS River Analysis System User's Manual. Version 5.0.

# FIGURES

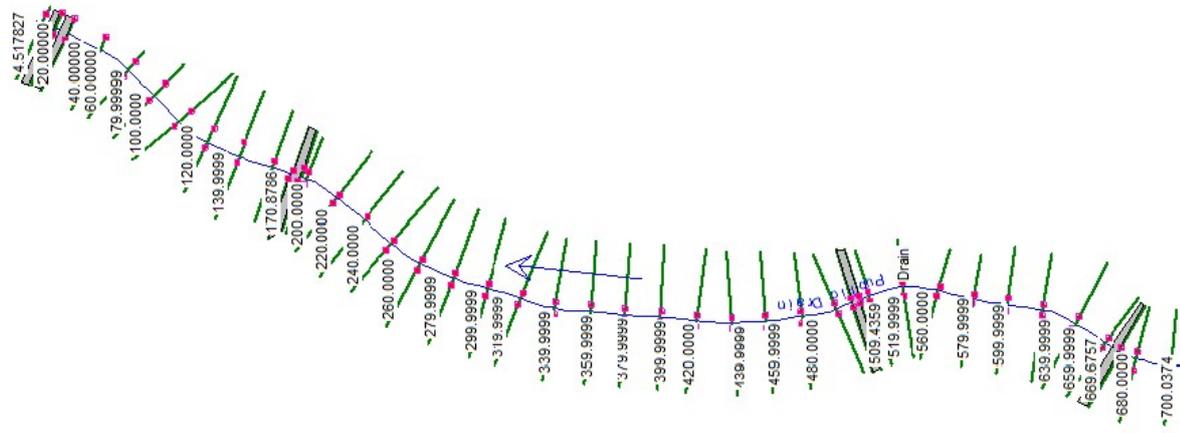


Figure 3.1 Model Schematic Exported from HEC-RAS

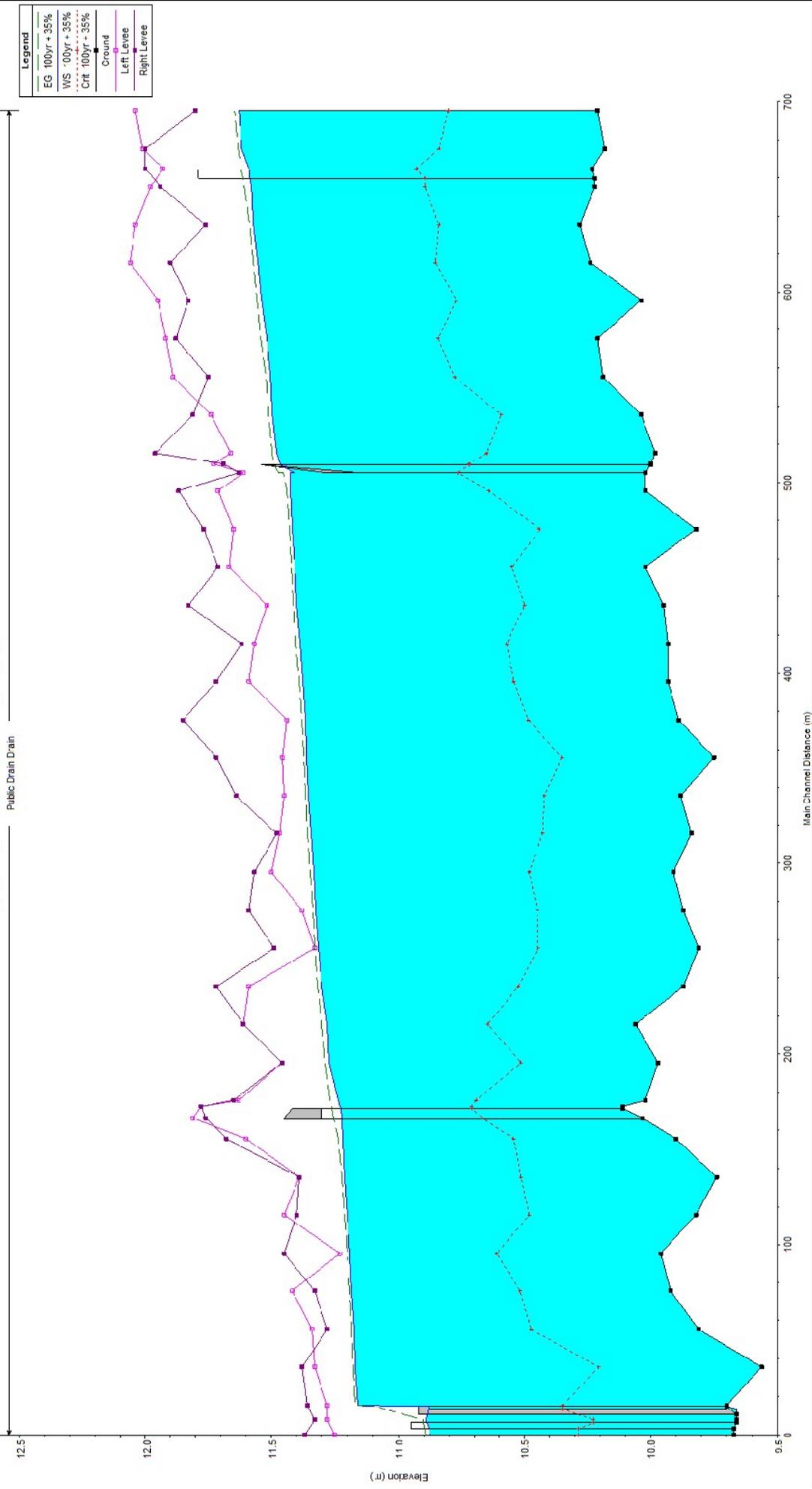


Figure 4.1: Long Profile of 100 year +35% Climate Change Model Run

# **TABLES**

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m2)	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%	0.41	0.17	11.63	10.8	11.65	0.000524	0.54	4.6	5.2	0.18
Drain	680	100yr + 35%	0.39	0.38	11.62	10.84	11.63	0.000619	0.57	4.33	5.11	0.2
Drain	669.6757	100yr + 35%	0.34	0.41	11.59	10.93	11.62	0.001524	0.81	3.06	3.8	0.29
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%	0.4	0.36	11.58	10.89	11.61	0.001097	0.74	3.34	3.69	0.25
Drain	639.9999	100yr + 35%	0.47	0.19	11.57	10.84	11.59	0.000511	0.53	4.65	5.45	0.18
Drain	620	100yr + 35%	0.51	0.35	11.55	10.85	11.57	0.000775	0.63	3.95	4.8	0.22
Drain	599.9999	100yr + 35%	0.41	0.29	11.54	10.77	11.56	0.000658	0.6	4.16	4.67	0.2
Drain	579.9999	100yr + 35%	0.39	0.36	11.52	10.84	11.54	0.000985	0.68	3.62	4.53	0.24
Drain	560	100yr + 35%	0.39	0.25	11.5	10.78	11.52	0.00079	0.64	3.86	4.44	0.22
Drain	540	100yr + 35%	0.24	0.31	11.5	10.59	11.51	0.000456	0.51	4.83	5.23	0.17
Drain	519.9999	100yr + 35%	0.18	0.48	11.48	10.65	11.5	0.000714	0.62	4.03	4.4	0.21
Drain	514.1835	100yr + 35%	0.26	0.22	11.47	10.72	11.49	0.001125	0.73	3.4	3.93	0.25
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%	0.18	0.2	11.43	10.76	11.46	0.001282	0.75	3.29	4.21	0.27
Drain	499.9999	100yr + 35%	0.29	0.45	11.42	10.64	11.44	0.000601	0.58	4.29	4.79	0.19
Drain	480	100yr + 35%	0.23	0.35	11.42	10.44	11.43	0.000341	0.46	5.34	5.35	0.15
Drain	459.9999	100yr + 35%	0.26	0.3	11.41	10.55	11.42	0.000461	0.52	4.78	5.27	0.17
Drain	439.9999	100yr + 35%	0.12	0.43	11.4	10.5	11.41	0.000463	0.52	4.78	5.11	0.17
Drain	420	100yr + 35%	0.18	0.23	11.39	10.57	11.4	0.000566	0.56	4.41	4.84	0.19
Drain	399.9999	100yr + 35%	0.21	0.34	11.38	10.54	11.39	0.000559	0.55	4.48	5.11	0.19
Drain	379.9999	100yr + 35%	0.07	0.48	11.37	10.49	11.38	0.000478	0.52	4.74	5.21	0.17
Drain	359.9999	100yr + 35%	0.1	0.36	11.36	10.35	11.37	0.000289	0.42	5.84	6.13	0.14
Drain	339.9999	100yr + 35%	0.1	0.29	11.35	10.42	11.37	0.000384	0.48	5.19	5.55	0.16
Drain	319.9999	100yr + 35%	0.13	0.14	11.34	10.43	11.36	0.000442	0.49	5.05	5.88	0.17
Drain	299.9999	100yr + 35%	0.17	0.23	11.33	10.48	11.35	0.000471	0.51	4.9	5.77	0.18
Drain	279.9999	100yr + 35%	0.05	0.26	11.33	10.45	11.34	0.00046	0.5	4.99	5.91	0.17
Drain	260	100yr + 35%	0.01	0.17	11.32	10.44	11.33	0.000519	0.5	4.93	6.39	0.18
Drain	240	100yr + 35%	0.29	0.42	11.3	10.52	11.32	0.00055	0.55	4.5	5.2	0.19
Drain	220	100yr + 35%	0.33	0.33	11.28	10.64	11.3	0.000995	0.67	3.69	4.97	0.25
Drain	200	100yr + 35%	0.19	0.19	11.27	10.51	11.29	0.000533	0.54	4.6	5.37	0.19
Drain	179.9999	100yr + 35%	0.39	0.41	11.24	10.69	11.27	0.001368	0.78	3.19	4.29	0.29
Drain	176.7426	100yr + 35%	0.55	0.55	11.23	10.71	11.26	0.001542	0.81	3.04	4.16	0.3
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%	0.59	0.54	11.22	10.67	11.25	0.001353	0.78	3.17	4.13	0.29
Drain	159.9999	100yr + 35%	0.38	0.46	11.22	10.54	11.24	0.000717	0.59	4.18	5.45	0.22
Drain	139.9999	100yr + 35%	0.19	0.19	11.21	10.51	11.22	0.000581	0.45	5.46	9.54	0.19
Drain	120	100yr + 35%	0.26	0.2	11.2	10.48	11.21	0.000456	0.42	5.84	9.51	0.17
Drain	100	100yr + 35%	0.04	0.27	11.19	10.61	11.2	0.000564	0.45	5.52	9.85	0.19
Drain	79.99999	100yr + 35%	0.25	0.15	11.18	10.52	11.19	0.00038	0.39	6.38	10.48	0.16
Drain	60	100yr + 35%	0.16	0.11	11.17	10.47	11.18	0.000424	0.42	5.9	9.29	0.17
Drain	40	100yr + 35%	0.16	0.21	11.17	10.21	11.18	0.000194	0.32	7.8	10.34	0.12
Drain	20	100yr + 35%	0.12	0.2	11.16	10.35	11.17	0.000638	0.47	5.32	9.47	0.2
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%	0.39	0.44	10.89	10.22	10.9	0.000673	0.51	4.9	8.15	0.21
Drain	7.988860	None	Bridge									
Drain	4.517827	100yr + 35%	0.38	0.5	10.87	10.28	10.89	0.001	0.63	3.93	6.2	0.25

Table 4.1: 1 in 100 Year + 35% Climate Change

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m2)	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%		0.27	0.03	11.77	10.8	11.78	0.000578	0.46	5.35	5.83
Drain	680	100yr + 35%		0.26	0.25	11.75	10.84	11.77	0.000649	0.49	5.06	5.51
Drain	669.6757	100yr + 35%		0.2	0.27	11.73	10.93	11.76	0.001561	0.68	3.63	4.25
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%		0.26	0.22	11.72	10.89	11.74	0.001165	0.64	3.88	3.91
Drain	639.9999	100yr + 35%		0.33	0.05	11.71	10.84	11.72	0.000528	0.46	5.43	5.77
Drain	620	100yr + 35%		0.37	0.21	11.69	10.85	11.71	0.000795	0.53	4.64	5.2
Drain	599.9999	100yr + 35%		0.27	0.15	11.68	10.77	11.69	0.000728	0.51	4.83	5.25
Drain	579.9999	100yr + 35%		0.26	0.22	11.66	10.84	11.68	0.00102	0.58	4.29	5.07
Drain	560	100yr + 35%		0.25	0.11	11.64	10.78	11.66	0.000886	0.55	4.5	5.09
Drain	540	100yr + 35%		0.11	0.18	11.63	10.59	11.64	0.000499	0.45	5.56	5.64
Drain	519.9999	100yr + 35%		0.05	0.35	11.63	10.65	11.63	0.00078	0.53	4.64	4.77
Drain	514.1835	100yr + 35%		0.13	0.09	11.6	10.72	11.62	0.001247	0.62	3.98	4.55
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%		0.07	0.09	11.54	10.76	11.57	0.001421	0.65	3.82	4.72
Drain	499.9999	100yr + 35%		0.17	0.33	11.54	10.64	11.55	0.000692	0.51	4.86	5.16
Drain	480	100yr + 35%		0.12	0.24	11.53	10.44	11.54	0.000427	0.41	5.98	6.03
Drain	459.9999	100yr + 35%		0.15	0.19	11.52	10.55	11.53	0.000532	0.46	5.41	5.58
Drain	439.9999	100yr + 35%		0.01	0.32	11.51	10.5	11.52	0.000545	0.46	5.36	5.45
Drain	420	100yr + 35%		0.07	0.12	11.5	10.57	11.51	0.00069	0.5	4.96	5.41
Drain	399.9999	100yr + 35%		0.11	0.24	11.48	10.54	11.49	0.000651	0.49	5.04	5.45
Drain	379.9999	100yr + 35%		-0.04	0.37	11.48	10.49	11.49	0.000151	0.25	16.18	42.49
Drain	359.9999	100yr + 35%		-0.02	0.24	11.48	10.35	11.48	0.000114	0.22	17.53	42.81
Drain	339.9999	100yr + 35%		-0.03	0.16	11.48	10.42	11.48	0.000174	0.27	14.77	42.88
Drain	319.9999	100yr + 35%		0.01	0.02	11.46	10.43	11.47	0.000531	0.43	5.81	6.92
Drain	299.9999	100yr + 35%		0.05	0.12	11.45	10.48	11.46	0.000545	0.44	5.62	6.51
Drain	279.9999	100yr + 35%		-0.07	0.14	11.45	10.45	11.45	0.000143	0.23	16.66	42.65
Drain	260	100yr + 35%		-0.12	0.04	11.45	10.44	11.45	0.000161	0.24	15.93	42.89
Drain	240	100yr + 35%		0.16	0.29	11.43	10.52	11.44	0.000588	0.48	5.2	5.53
Drain	220	100yr + 35%		0.2	0.2	11.41	10.64	11.43	0.001254	0.56	4.42	6.77
Drain	200	100yr + 35%		0.06	0.06	11.4	10.51	11.41	0.00058	0.47	5.31	5.79
Drain	179.9999	100yr + 35%		0.26	0.28	11.37	10.69	11.39	0.00137	0.66	3.78	4.62
Drain	176.7426	100yr + 35%		0.42	0.42	11.36	10.71	11.39	0.001585	0.68	3.63	4.74
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%		0.47	0.42	11.34	10.67	11.36	0.001448	0.68	3.66	4.39
Drain	159.9999	100yr + 35%		0.27	0.35	11.33	10.54	11.34	0.000792	0.51	4.83	5.98
Drain	139.9999	100yr + 35%		0.08	0.08	11.32	10.51	11.33	0.000564	0.38	6.58	10.61
Drain	120	100yr + 35%		0.15	0.09	11.31	10.48	11.32	0.000474	0.36	6.97	10.86
Drain	100	100yr + 35%		-0.08	0.15	11.31	10.61	11.31	0.000216	0.24	14.26	40.75
Drain	79.99999	100yr + 35%		0.13	0.03	11.3	10.52	11.3	0.00037	0.32	7.66	11.54
Drain	60	100yr + 35%		0.04	-0.01	11.29	10.47	11.3	0.00044	0.35	7.06	10.85
Drain	40	100yr + 35%		0.04	0.09	11.29	10.21	11.29	0.000211	0.27	9.05	11.32
Drain	20	100yr + 35%		0	0.08	11.28	10.35	11.28	0.000296	0.25	13.27	43.1
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%		0.27	0.32	11.01	10.22	11.02	0.000652	0.41	5.98	9.35
Drain	7.988860	None	Bridge									
Drain	4.517827	100yr + 35%		0.26	0.38	10.99	10.28	11	0.001002	0.53	4.7	6.88

Table 4.2: 1 in 100 Year + 35% Climate Change Plus 25% Increase in Mannings

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m2)	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%	0.4	0.16	11.64	10.8	11.66	0.000509	0.53	4.66	5.25	0.18
Drain	680	100yr + 35%	0.38	0.37	11.63	10.84	11.65	0.000597	0.56	4.39	5.14	0.2
Drain	669.6757	100yr + 35%	0.33	0.4	11.6	10.93	11.63	0.001466	0.8	3.1	3.84	0.28
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%	0.39	0.35	11.59	10.89	11.62	0.001059	0.73	3.39	3.71	0.24
Drain	639.9999	100yr + 35%	0.46	0.18	11.58	10.84	11.6	0.000494	0.53	4.71	5.48	0.18
Drain	620	100yr + 35%	0.49	0.33	11.57	10.85	11.58	0.000748	0.62	4	4.83	0.22
Drain	599.9999	100yr + 35%	0.4	0.28	11.55	10.77	11.57	0.000636	0.59	4.21	4.7	0.2
Drain	579.9999	100yr + 35%	0.39	0.35	11.53	10.84	11.55	0.000948	0.67	3.68	4.58	0.24
Drain	560	100yr + 35%	0.37	0.23	11.52	10.78	11.54	0.00076	0.63	3.92	4.47	0.22
Drain	540	100yr + 35%	0.23	0.3	11.51	10.59	11.52	0.00044	0.51	4.9	5.27	0.17
Drain	519.9999	100yr + 35%	0.17	0.47	11.49	10.65	11.51	0.000689	0.61	4.08	4.44	0.2
Drain	514.1835	100yr + 35%	0.25	0.21	11.48	10.72	11.5	0.001089	0.72	3.45	3.99	0.25
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%	0.18	0.2	11.43	10.76	11.46	0.00126	0.75	3.32	4.23	0.27
Drain	499.9999	100yr + 35%	0.28	0.44	11.43	10.64	11.45	0.000594	0.57	4.31	4.8	0.19
Drain	480	100yr + 35%	0.23	0.35	11.42	10.44	11.44	0.000338	0.46	5.36	5.36	0.15
Drain	459.9999	100yr + 35%	0.26	0.3	11.41	10.55	11.43	0.000456	0.51	4.83	5.28	0.17
Drain	439.9999	100yr + 35%	0.12	0.43	11.4	10.5	11.42	0.000458	0.52	4.8	5.12	0.17
Drain	420	100yr + 35%	0.18	0.23	11.39	10.57	11.41	0.000561	0.56	4.43	4.86	0.19
Drain	399.9999	100yr + 35%	0.21	0.34	11.38	10.54	11.4	0.000552	0.55	4.5	5.12	0.19
Drain	379.9999	100yr + 35%	0.07	0.48	11.37	10.49	11.39	0.000472	0.52	4.77	5.22	0.17
Drain	359.9999	100yr + 35%	0.09	0.35	11.37	10.35	11.38	0.000287	0.42	5.86	6.16	0.14
Drain	339.9999	100yr + 35%	0.09	0.28	11.36	10.42	11.37	0.000379	0.48	5.21	5.56	0.16
Drain	319.9999	100yr + 35%	0.12	0.13	11.35	10.43	11.36	0.000438	0.49	5.07	5.92	0.17
Drain	299.9999	100yr + 35%	0.16	0.23	11.34	10.48	11.35	0.000466	0.5	4.93	5.8	0.17
Drain	279.9999	100yr + 35%	0.05	0.26	11.34	10.45	11.34	0.000456	0.49	5.01	5.95	0.17
Drain	260	100yr + 35%	0.01	0.17	11.32	10.44	11.33	0.000515	0.5	4.96	6.44	0.18
Drain	240	100yr + 35%	0.28	0.41	11.31	10.52	11.32	0.000543	0.55	4.52	5.21	0.19
Drain	220	100yr + 35%	0.33	0.33	11.28	10.64	11.31	0.00098	0.67	3.71	4.99	0.25
Drain	200	100yr + 35%	0.18	0.18	11.28	10.51	11.29	0.000526	0.54	4.62	5.38	0.19
Drain	179.9999	100yr + 35%	0.39	0.41	11.24	10.69	11.27	0.001348	0.77	3.21	4.3	0.29
Drain	176.7426	100yr + 35%	0.55	0.55	11.23	10.71	11.27	0.00152	0.81	3.06	4.18	0.3
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%	0.58	0.53	11.23	10.67	11.26	0.001335	0.78	3.18	4.14	0.28
Drain	159.9999	100yr + 35%	0.38	0.46	11.22	10.54	11.24	0.000712	0.59	4.19	5.46	0.22
Drain	139.9999	100yr + 35%	0.19	0.19	11.21	10.51	11.22	0.000577	0.45	5.47	9.56	0.19
Drain	120	100yr + 35%	0.26	0.2	11.2	10.48	11.21	0.000453	0.42	5.85	9.53	0.17
Drain	100	100yr + 35%	0.04	0.27	11.19	10.61	11.2	0.00056	0.45	5.54	9.87	0.19
Drain	79.99999	100yr + 35%	0.25	0.15	11.18	10.52	11.19	0.000378	0.39	6.39	10.5	0.16
Drain	60	100yr + 35%	0.16	0.11	11.17	10.47	11.18	0.000421	0.42	5.91	9.3	0.17
Drain	40	100yr + 35%	0.16	0.21	11.17	10.21	11.18	0.000194	0.32	7.81	10.35	0.12
Drain	20	100yr + 35%	0.12	0.2	11.16	10.35	11.17	0.000635	0.47	5.33	9.49	0.2
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%	0.39	0.44	10.89	10.22	10.9	0.00067	0.51	4.91	8.16	0.21
Drain	7.988860	None	Bridge									
Drain	4.517827	100yr + 35%	0.38	0.5	10.87	10.28	10.89	0.001	0.63	3.93	6.2	0.25

Table 4.1: 1 in 100 Year + 35% Climate Change Plus 25% Increase on Contraction and Expansion Losses

Reach	River Station	Profile	Left Freeboard (m)	Right Freeboard (m)	Water Surface Elevation (m)	Critical Water Surface (m)	Energy Grade Elevation (m)	Energy Grade Slope (m/m)	Velocity Channel (m/s)	Flow Area (m2)	Top Width (m)	Froude Number Channel
Drain	700.0374	100yr + 35%	0.25	0.01	11.79	10.87	11.81	0.00054	0.57	5.48	5.93	0.19
Drain	680	100yr + 35%	0.23	0.22	11.78	10.92	11.79	0.000603	0.6	5.17	5.57	0.2
Drain	669.6757	100yr + 35%	0.18	0.25	11.75	11.02	11.78	0.001474	0.84	3.69	4.3	0.29
Drain	664.9016	None	Bridge									
Drain	659.9999	100yr + 35%	0.24	0.2	11.74	10.97	11.77	0.001101	0.79	3.94	3.93	0.25
Drain	639.9999	100yr + 35%	0.31	0.03	11.73	10.9	11.75	0.000492	0.56	5.54	5.82	0.18
Drain	620	100yr + 35%	0.35	0.19	11.71	10.92	11.73	0.000743	0.65	4.74	5.25	0.22
Drain	599.9999	100yr + 35%	0.25	0.13	11.7	10.84	11.72	0.000685	0.63	4.94	5.35	0.21
Drain	579.9999	100yr + 35%	0.24	0.2	11.68	10.92	11.7	0.000951	0.71	4.39	5.15	0.24
Drain	560	100yr + 35%	0.23	0.09	11.66	10.85	11.68	0.000834	0.67	4.61	5.23	0.17
Drain	540	100yr + 35%	0.09	0.16	11.65	10.66	11.67	0.000463	0.54	5.7	5.72	0.17
Drain	519.9999	100yr + 35%	0.02	0.32	11.64	10.73	11.66	0.000727	0.65	4.75	4.83	0.21
Drain	514.1835	100yr + 35%	0.11	0.07	11.62	10.81	11.65	0.001169	0.76	4.08	4.64	0.26
Drain	511.7022	None	Bridge									
Drain	509.4359	100yr + 35%	0.07	0.09	11.54	10.85	11.58	0.001406	0.81	3.82	4.72	0.29
Drain	499.9999	100yr + 35%	0.17	0.23	11.54	10.71	11.56	0.00068	0.64	4.88	5.17	0.21
Drain	480	100yr + 35%	0.11	0.33	11.54	10.51	11.55	0.000418	0.52	6	6.06	0.17
Drain	459.9999	100yr + 35%	0.15	0.19	11.52	10.62	11.54	0.000521	0.57	5.42	5.59	0.19
Drain	439.9999	100yr + 35%	0.01	0.32	11.51	10.57	11.53	0.000534	0.58	5.38	5.46	0.19
Drain	420	100yr + 35%	0.07	0.12	11.5	10.64	11.52	0.000678	0.62	4.97	5.42	0.21
Drain	399.9999	100yr + 35%	0.11	0.23	11.48	10.61	11.5	0.000638	0.61	5.06	5.46	0.2
Drain	379.9999	100yr + 35%	-0.05	0.36	11.49	10.56	11.49	0.000143	0.3	16.48	42.5	0.1
Drain	359.9999	100yr + 35%	-0.03	0.23	11.49	10.42	11.49	0.000108	0.27	17.85	42.81	0.09
Drain	339.9999	100yr + 35%	-0.03	0.16	11.48	10.49	11.49	0.000164	0.33	15.07	42.88	0.1
Drain	319.9999	100yr + 35%	0	0.01	11.47	10.5	11.48	0.000521	0.53	5.83	6.95	0.19
Drain	299.9999	100yr + 35%	0.04	0.11	11.46	10.55	11.47	0.000535	0.55	5.64	6.53	0.19
Drain	279.9999	100yr + 35%	-0.08	0.13	11.46	10.52	11.46	0.000136	0.28	16.93	42.67	0.1
Drain	260	100yr + 35%	-0.13	0.03	11.46	10.52	11.46	0.000153	0.29	16.21	42.91	0.1
Drain	240	100yr + 35%	0.16	0.29	11.43	10.6	11.45	0.000581	0.6	5.2	5.53	0.2
Drain	220	100yr + 35%	0.2	0.2	11.41	10.71	11.43	0.00124	0.7	4.4	6.71	0.28
Drain	200	100yr + 35%	0.06	0.06	11.4	10.58	11.42	0.000573	0.58	5.31	5.79	0.19
Drain	179.9999	100yr + 35%	0.27	0.29	11.36	10.77	11.4	0.001388	0.83	3.74	4.6	0.29
Drain	176.7426	100yr + 35%	0.43	0.43	11.35	10.78	11.39	0.001611	0.86	3.59	4.7	0.32
Drain	173.7661	None	Bridge									
Drain	170.8786	100yr + 35%	0.48	0.43	11.33	10.74	11.37	0.001477	0.86	3.62	4.37	0.3
Drain	159.9999	100yr + 35%	0.28	0.36	11.32	10.61	11.35	0.0008	0.65	4.79	5.95	0.23
Drain	139.9999	100yr + 35%	0.08	0.08	11.32	10.59	11.33	0.000568	0.47	6.54	10.57	0.19
Drain	120	100yr + 35%	0.15	0.09	11.31	10.55	11.32	0.000476	0.45	6.93	10.82	0.18
Drain	100	100yr + 35%	-0.08	0.15	11.31	10.67	11.31	0.000218	0.3	14.17	40.74	0.12
Drain	79.99999	100yr + 35%	0.13	0.03	11.3	10.58	11.3	0.000372	0.41	7.61	11.51	0.16
Drain	60	100yr + 35%	0.04	-0.01	11.29	10.55	11.3	0.000443	0.44	7	10.7	0.17
Drain	40	100yr + 35%	0.05	0.1	11.28	10.27	11.29	0.000211	0.34	9	11.29	0.12
Drain	20	100yr + 35%	0.01	0.09	11.27	10.43	11.28	0.000678	0.48	6.5	11.86	0.21
Drain	19.05487	None	Culvert									
Drain	12.28586	100yr + 35%	0.26	0.31	11.02	10.29	11.03	0.000636	0.52	6.02	9.38	0.21
Drain	7.988860	None	Bridge									
Drain	4.517827	100yr + 35%	0.26	0.38	10.99	10.36	11.01	0.001002	0.66	4.68	6.86	0.26

Table 4.4: 1 in 100 Year + 35% Climate Change Plus 25% Increase on Peak Flows

# APPENDICES

VERSION	"FEH CD-ROM"	Version	3 exported a	11:12:03 GMT	Thu	23-Feb-17
CATCHMENT	GB	543800	262900	TL 43800	62900	
CENTROID	GB	543356	261519	TL 43356	61519	
AREA		3.2725				
ALTBAR		18				
ASPBAR		35				
ASPVAR		0.55				
BFIHOST		0.542				
DPLBAR		1.89				
DPSBAR		7				
FARL		0.983				
FPEXT		0.272				
FPDBAR		1.159				
FPLOC		0.911				
LDP		3.72				
PROPWET		0.26				
RMED-1H		10.8				
RMED-1D		28.7				
RMED-2D		34.6				
SAAR		546				
SAAR4170		553				
SPRHOST		43.27				
URBCONC1990		0.335				
URBEXT1990		0.068				
URBLOC1990		1.012				
URBCONC2000		0.771				
URBEXT2000		0.1276				
URBLOC2000		1.199				
C		-0.026				
D1		0.31487				
D2		0.24606				
D3		0.2844				
E		0.31745				
F		2.44653				
C(1 km)		-0.026				
D1(1 km)		0.316				
D2(1 km)		0.248				
D3(1 km)		0.279				
E(1 km)		0.318				
F(1 km)		2.442				

# PLANS



























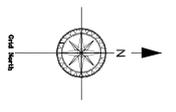
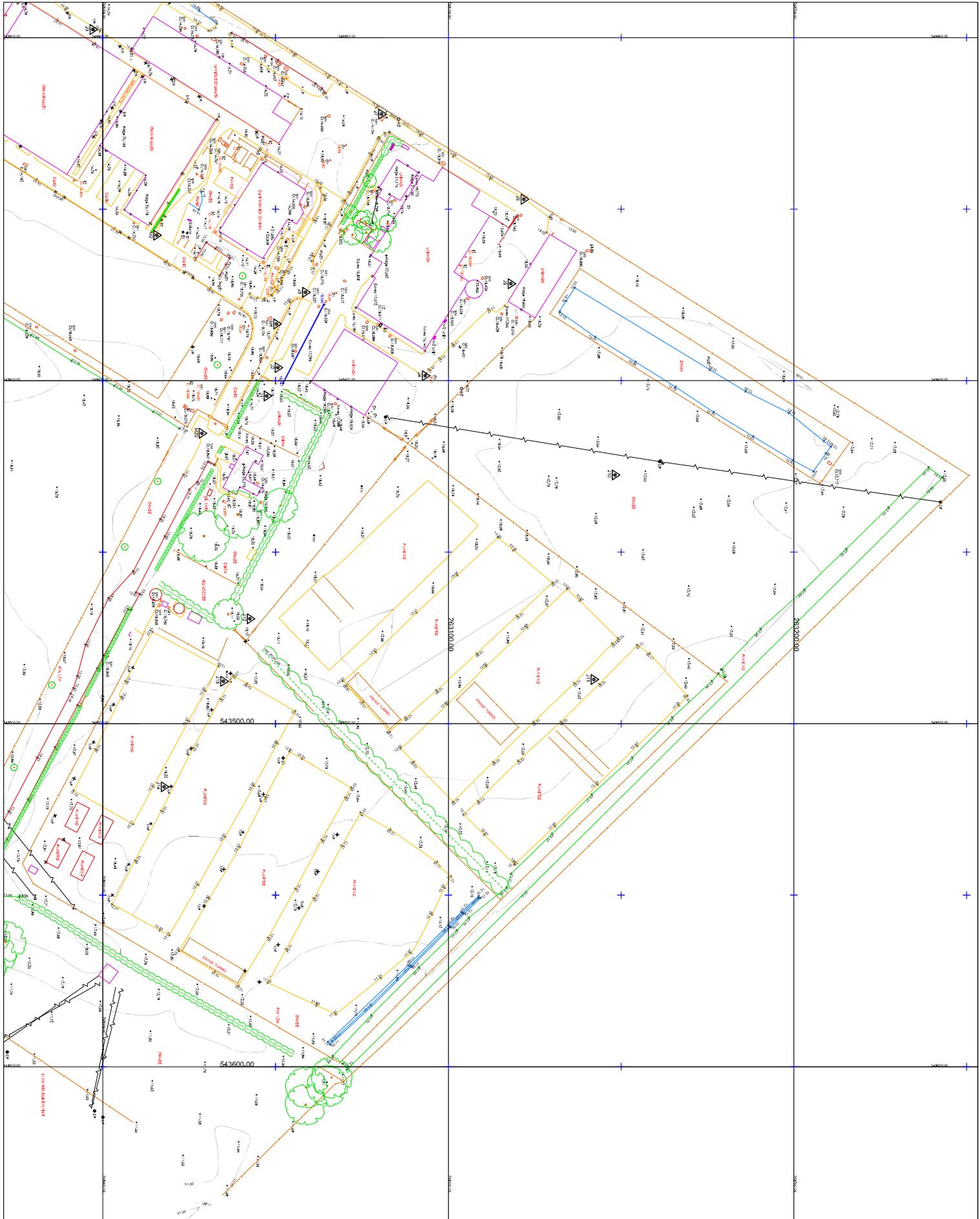












SHEET TO ORGANISE SURVEY  
 OPERATIONS PLANNED FROM  
 DISTANCE MEASUREMENTS  
**Co-Ordinates**

54304.445	55347.489	13.8881
54342.507	55314.238	12.7465
54486.388	55342.507	13.2773
54347.612	55304.884	13.1246
54338.185	55300.613	13.5770
54347.983	55307.846	13.3323
54333.024	55305.444	14.0507
54340.488	55351.924	15.4242
54351.328	55358.072	15.1178
54329.701	55351.231	14.7589
54337.046	55355.032	13.6260
54327.827	55354.570	13.5289
54338.547	55303.270	13.4241
54327.710	55311.830	13.4915
54322.283	55309.438	14.1501
54326.917	55359.032	14.2053
54323.007	55301.528	13.2489
54338.488	55300.228	13.7970
54341.288	55307.914	13.7974
54342.035	55309.888	14.4054
54343.072	55351.124	13.1814
54338.827	55359.111	11.9519
54331.177	55352.448	14.2814
54338.942	55357.241	14.2813
54324.196	55352.488	14.4215
54327.193	55353.571	14.2523

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09	[Symbol]	1:500	
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49	[Symbol]	1:500	
50	[Symbol]	1:500	

Date: October 2016  
 Scale: 1:500  
 Drawing Number: A15751 B/500/01  
 Topographical  
 Client: Smith Evans Architects  
 Project: NAB Park Farm  
 Villa Road, Milton

Shaded Site  
**ALS** EST 1978  
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